



# Deflection of Beams

## Lecture 4 – Statically Indeterminate Problems

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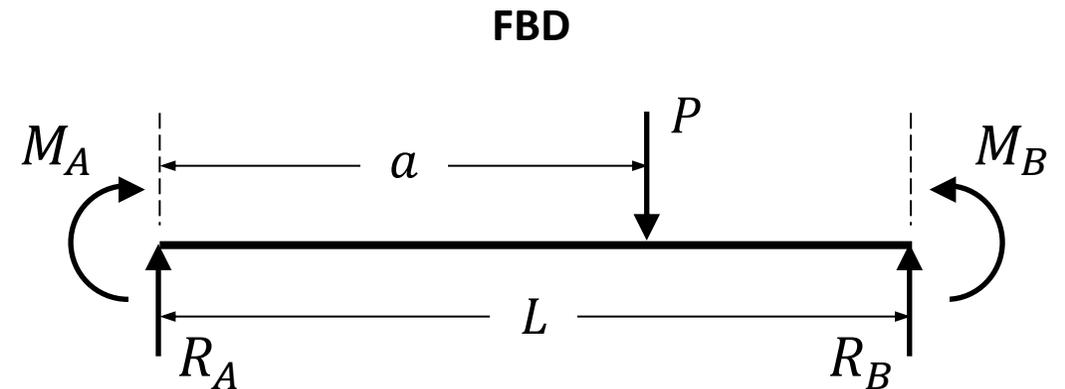
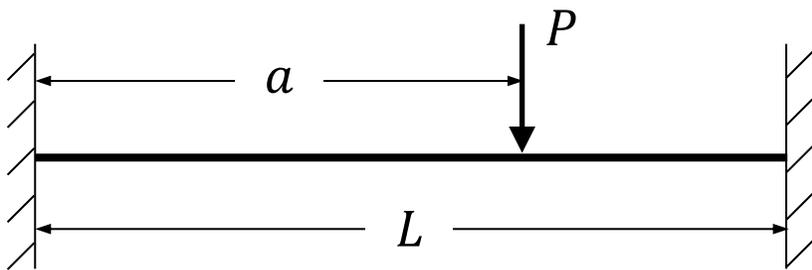
## Learning Outcomes

1. Know how to derive the differential equation of the elastic line (i.e. deflection curve) of a beam (synthesis);
2. Employ Macaulay's method, also called the method of singularities, to determine bending moment expressions for beams where there are discontinuities in the bending moment distribution arising from discontinuous loading (application);
3. Be able to solve this equation by successive integration in order to yield the slope,  $\frac{dy}{dx}$ , and the deflection,  $y$ , of a beam at any position,  $x$ , along its span (application);
4. Recognise and use different singularity functions in the bending moment expression, relating to different loading conditions, including point loads, uniformly distributed loads and point bending moments (comprehension);
5. Employ Macaulay's method for statically indeterminate beam problems (application).

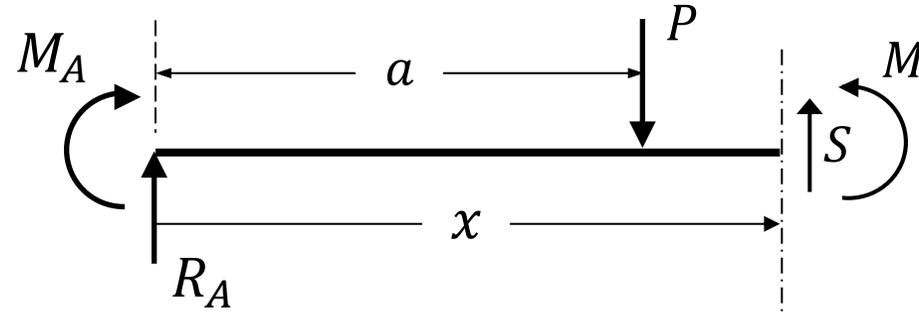
# Statically Indeterminate Problems

Macaulay's method can also be used to solve for the slopes and deflections of statically indeterminate problems.

A beam is statically indeterminate if the reaction forces and/or bending moments cannot be determined by the equations of statics alone.



Taking the left-hand end as the origin and drawing a free body diagram of the beam sectioned after the discontinuity:



Taking moments about the section position in order to determine an expression for the bending moment,  $M$ :

$$M = R_A x + M_A - P \langle x - a \rangle$$

Substituting this into the 2<sup>nd</sup> order differential equation of the elastic line (equation (8) from lecture 1):

$$EI \frac{d^2 y}{dx^2} = R_A x + M_A - P \langle x - a \rangle$$

This 2<sup>nd</sup> order differential expression for the clamped-clamped beam can then be integrated with respect to  $x$  to give the slope,  $\frac{dy}{dx}$ , and integrated again to give the deflection,  $y$ , at any position  $x$ , along the length of the beam.

Integrating with respect to  $x$ :

$$EI \frac{dy}{dx} = \frac{R_A x^2}{2} + M_A x - \frac{P(x-a)^2}{2} + A$$

Integrating with respect to  $x$  again:

$$EIy = \frac{R_A x^3}{6} + \frac{M_A x^2}{2} - \frac{P(x-a)^3}{6} + Ax + B$$

These expressions for slope and deflection contain four unknowns, namely the reactions  $M_A$  and  $R_A$  and the integration constants  $A$  and  $B$ . In this case, we can apply the four boundary conditions to these expressions to solve for these four unknowns.

$$\text{BC1: at } x = 0, y = 0$$

$$\text{BC2: at } x = 0, \frac{dy}{dx} = 0$$

$$\text{BC3: at } x = L, y = 0$$

$$\text{BC4: at } x = L, \frac{dy}{dx} = 0$$

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